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8. Jcko Nugroho, Dhemi Harlan, Hendra Achiari, "Preliminary Study on Scouring at Parallel River Crossing Pipelines".





## LEMBAR PENGESAHAN

Judul : The Application of Combined Gravity-Multinomial Logit Model and  
Equilibrium Assignment in Artificial Network

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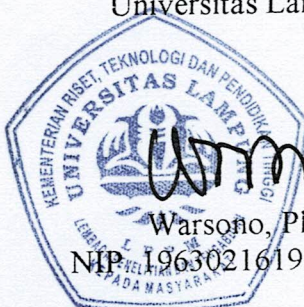
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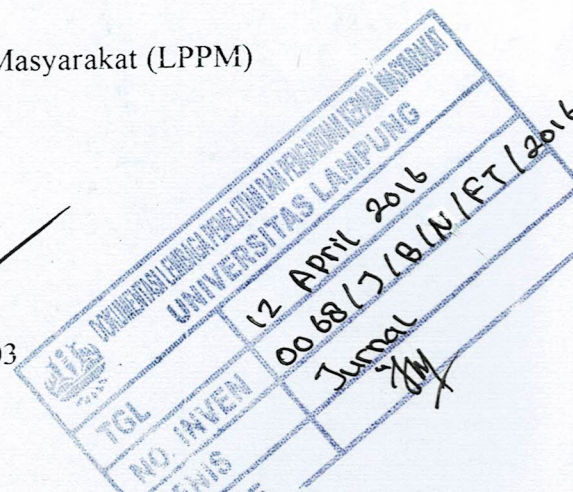
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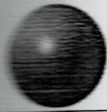


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## The Application of the Combined Model of Gravity-Multinomial Logit and the Assignment of Equilibrium Trip in Simple Network

Rahayu Sulistyorini<sup>1)</sup> and Ofyar Z. Tamin<sup>2)</sup>

### ABSTRACT

Transportation problems generally require Origin-Destination Matrix (ODM) as their main input that represents travel pattern on a planning area. One method used to obtain the ODM is the ODM estimation method based on traffic data that belong to the category of sufficiently effective and economic, unconventional method. Model developed in this research combine the stages of movement distribution, the assignment of modes and trips, so that this model would be able to simultaneously estimate the ODM for different transportation modes. Some previous researches still advocate "All of Nothing" mode that was unrealistic for some road networks in urban areas for their ignorance of the effects of congestion and perceptual diversities in considering trips options. This research adopts gravity movement dispersion model, multinomial logit mode assignments model and balanced route assignment model. The estimation model developed to calibrate these models using traffic data are as follows: least square (LS). The developed model would be subsequently tested using simple dummy data. Results suggest that the model was successfully run through C++ language program. It is expected that the application on the real road networks un Bandung City will use EMME/2 software.

Keywords: Combination model of movement dispersion and Mode Selection, Equilibrium Assignment, Gravity, Multinomial Logit, nonlinear least square

### 1. INTRODUCTION

The majority of techniques and methods to solve transportation problems require ODM information as base line information to plot transportation needs levels (Tamin, 2000), that could be obtained through conventional method and unconventional method. The main constraint in using conventional method is that it is lengthy and requires significant costs with high error and disturbance rates on the part of the traveling actors. To cope this constrain, we need inexpensive method in terms of data and time so that "unconventional methods" were formulated. This method is inexpensive in term that it only requires traffic flow data.

The Four-Stage Transportation Planning model is gradual processes from several separated and sequential sub-models. This model could be simplified to meet transportation planning needs in areas with limited times and financial resources. Currently, combined models are increasingly developed. These combined model are essentially the aggregate sum of some stages

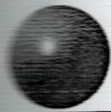
of the sub-models, either partially or simultaneously. Some previous researches are still in an "All of Nothing" state that was unrealistic for some road networks in urban areas for their ignorance of the effects of congestion and perceptual diversities in considering routes options. Model developed in this research combine the stage of movement dispersion, modes assignment and route assignments. Movement dispersion model adopted is the gravity model; whereas the mode assignment model adopted is the multinomial logit (ML) model, and the route assignment model adopted is equilibrium route assignment model.

Cascetta and Russo (1997) classified Origin-Destination Matrix (ODM) into 2 (two) categories, i.e., the estimation resulting in Direct Origin-Destination Matrix and the estimation of parameters of transportation need model. In estimating the parameters of transportation need model, in addition to the ODM, other resulted output is parameters of transportation needs model ( $\alpha$  and  $\beta$ ).

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**II. PREVIOUS RESEARCH ON THE ESTIMATION OF TRAFFIC CURRENT BASED ODM**

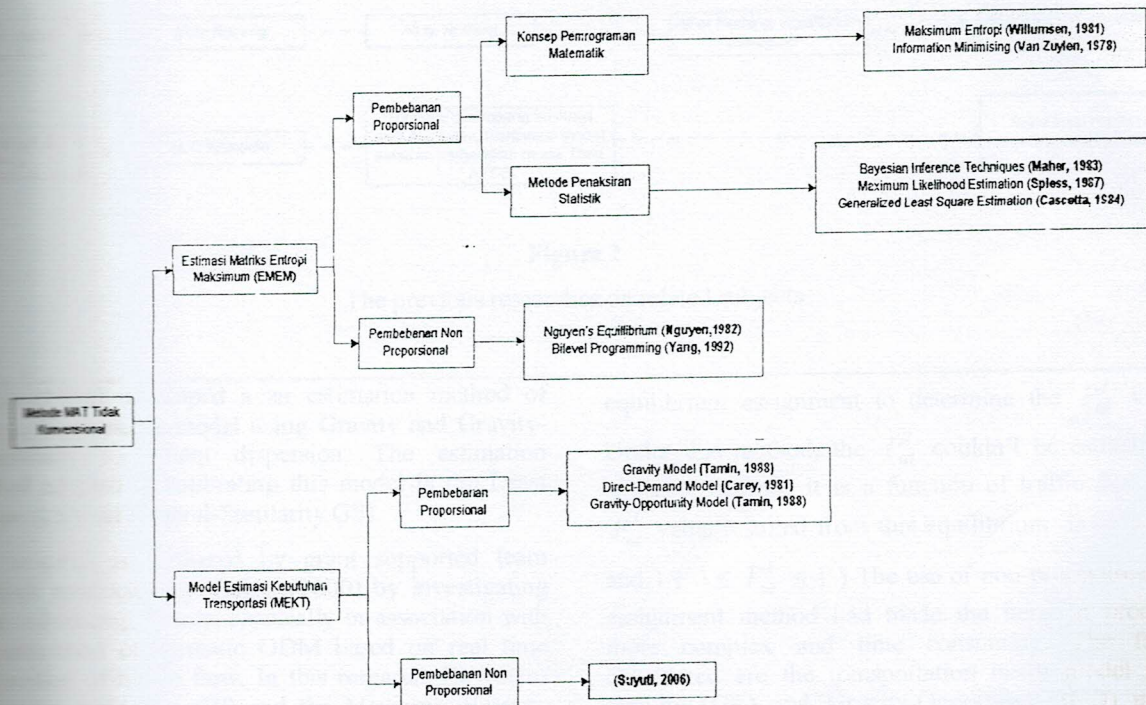
Some previous researches investigated the degree of accuracy of the ODM based on unconventional method (see Figure 1).

According to Yang et. al. (1992), the estimation of the ODM based on the information of traffic current has been performed by many researchers. The methods adopted are as follows: Maximum-Entropy (Van Zuylen and Willumsen, 1980), Maximum-Similarity (Spess, 1987), General-Least-Square (Cascett, 1984) and Bayesian Inferential Estimation (Maher, 1983, in Tamin, 2000). In general, these models are aimed at minimizing the differences between the traffic currents and the ODM in terms of observational results and the estimation outputs. In this case, the trip assignment mode is used as a function linking the estimation

output's traffic currents and the estimation output's ODM.

Van Zuylen and Willumsen (1981) generated the ODM estimation model of traffic current through the use of the Maximum-Entropy approach. Cascetta (1984) developed the General-Least-Square (GLS) estimation model aimed at estimating the ODM by using the combination of the model estimation and traffic flow data. In the meanwhile, Maher (1983) cited by Tamin (2000) as having developed the ODM estimation method of traffic flow through Bayesian statistical approach.

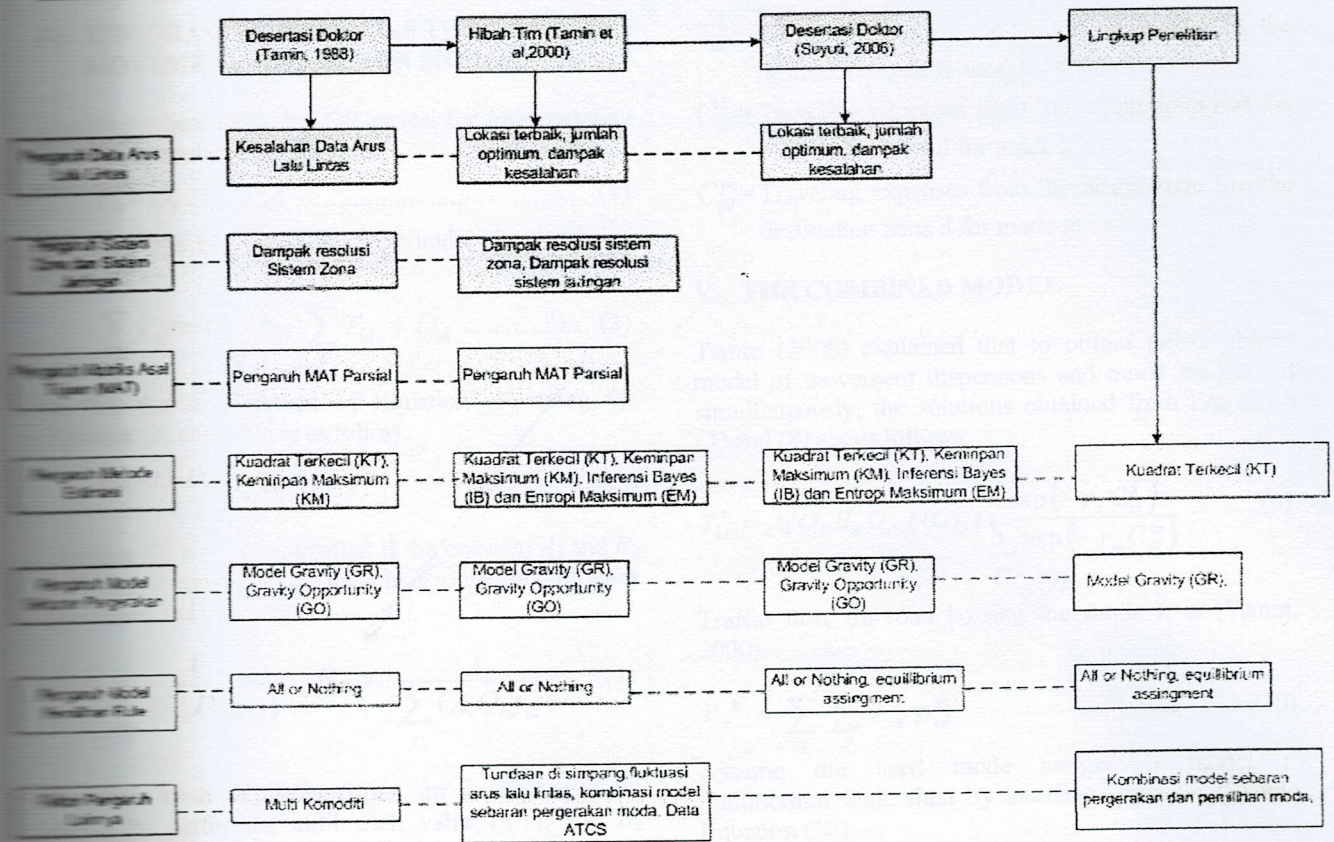
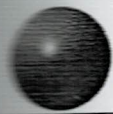
Tamin (1988) conducted studies to estimate the parameters of transportation needs model using information of traffic flow. Under his study, the  $P_{id}^l$  value for each l columns is assumed to be constant and independent of the changes on the estimated ODM. The trip assignment methods adopted is all-or-nothing (see Figure 2).



**Figure 1**

Transportation model based on Traffic Flow Data  
 (Source: Suyuti, 2006, adapted from Tamin, et al., 2002)





**Figure 2**

The previous researches on related subjects

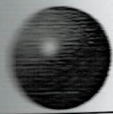
Tamin (1988) developed a an estimation method of transportation needs model using Gravity and Gravity-Opportunity movement dispersion, The estimation method adopted in calibrating this model is the Least Square (LS) and General-Similarity GS).

The research is furthered by grant supported team research conducted by Tamin (2000) by investigating other influencing factors, especially in association with the estimation of dynamic ODM based on real time information of traffic flow, In this research, Bayesian-Inferential estimation (BI) and the Maximum-Entropy are introduced. In addition, other factors addressed are: the best location if traffic flow data, the impact of delay on the junctions, the impacts of traffic flow fluctuations and the combination of the models of mode assignment movement dispersion.

Based on these conditions, Suyuti (2006) developed the non-proportional trip assignment technique such as

equilibrium assignment to determine the  $P_{id}^l$  values. Under this method, the  $P_{id}^l$  couldn't be estimated in advance, because it is a function of traffic flow. The  $P_{id}^l$  value resulted from this equilibrium is between 0 and 1 ( $0 \leq P_{id}^l \leq 1$ ) The use of non-proportional trip assignment method had made the iteration processes more complex and time consuming. The factors considered are the transportation needs model under Gravity (GR) and Gravity-Opportunity (GO) model, with least-square (LS), maximum-similarity (MS), Bayesian Inferential (BI) and the Maximum Entropy (ME) estimation methods. The trip assignment methods adopted are all-or-nothing and equilibrium assignment and the impacts of traffic flow data (location, the size of data and the error level of traffic flow data).





### III. THE GRAVITY MODEL AS THE MOVEMENT DISPERSION MODEL

In mathematical form, the GR model for transportation could be formulated as:

$$T_{id} = O_i \cdot D_d \cdot f(C_{id}) \dots\dots\dots (1)$$

The Equation (1) could be used under the following constraints:

$$\sum_d T_{id} = O_i \text{ dan } \sum_i T_{id} = D_d \dots\dots\dots (2)$$

So that the development of Equation (1) using the Equation (2) constrain is as follow:

$$T_{id} = O_i \cdot D_d \cdot A_i \cdot B_d \cdot f(C_{id}) \dots\dots\dots (3)$$

Equation (3) would be satisfied if the constant  $A_i$  and  $B_d$  (or balancer constants) are used in association with each generation and pull zone.

$$A_i = \frac{1}{\sum_d (B_d \cdot D_d \cdot f_{id})} \quad B_d = \frac{1}{\sum_i (A_i \cdot O_i \cdot f_{id})} \dots\dots (4)$$

To obtain both of these values, an iteration process should be performed until each value of  $A_i$  and  $B_d$  generate certain (convergent) value.

Hyman (1969) in Tamin (2000) said that there are three types of additional functions that we could use in the Gravity Model, namely:

- quadratic function:  $f(C_{id}) = C_{id}^{-\alpha} \dots\dots\dots (5)$

- exponential function:  $f(C_{id}) = e^{-\beta C_{id}} \dots\dots\dots (6)$

- Turner Function:  $f(C_{id}) = C_{id}^\alpha \cdot e^{-\beta C_{id}} \dots\dots\dots (7)$

### IV. THE MODEL OF MULTINOMIAL LOGIT AS MODE ASSIGNMENT MODEL

The mode assignment model is aimed at knowing the proportion of peoples who will use each mode. We could formulate the multinomial logit as follow:

$$T_{id}^k = T_{id} \frac{\exp(-\beta C_{id}^k)}{\sum_m \exp(-\beta C_{id}^m)} \dots\dots\dots (9)$$

$T_{id}^k$  = the total movements from the origin zone i to the destination zone d.

$T_{id}^k$  = total movement from the origin zone i to the destination zone d using mode k

$C_{id}^k$  = Traveling expenses from the origin zone i to the destination zone d for mode k

$C_{id}^m$  = Traveling expenses from the origin zone i to the destination zone d for mode m

### V. THE COMBINED MODEL

Tamin (2000) explained that to obtain the combined model of movement dispersions and mode assignment simultaneously, the solutions obtained from Equations (3) and (8) are as follows:

$$T_{id}^k = A_i \cdot O_i \cdot B_d \cdot D_d \cdot f(C_{id}) \frac{\exp(-\gamma_k C_{id}^k)}{\sum_m \exp(-\gamma_m C_{id}^m)} \dots\dots\dots (9)$$

Traffic flow on road l using the mode k is (Tamin, 2000):

$$V_l^k = \sum_i \sum_d T_{id}^k \cdot p_{id}^{lk} \dots\dots\dots (10)$$

Assume the used mode assignment model is multinomial logit, then by inserting Equation (8) Into Equation (10):

$$V_l^k = \sum_i \sum_d T_{id} \frac{\exp(-\gamma_k C_{id}^k)}{\sum_m \exp(-\gamma_m C_{id}^m)} p_{id}^{lk} \dots\dots\dots (11)$$

Assume that inter-zone movements could be represented by a single model of transportation needs ( e.g., gravity) then we would obtain the estimation basic equation of the combined transportation model using traffic flow data as follows:

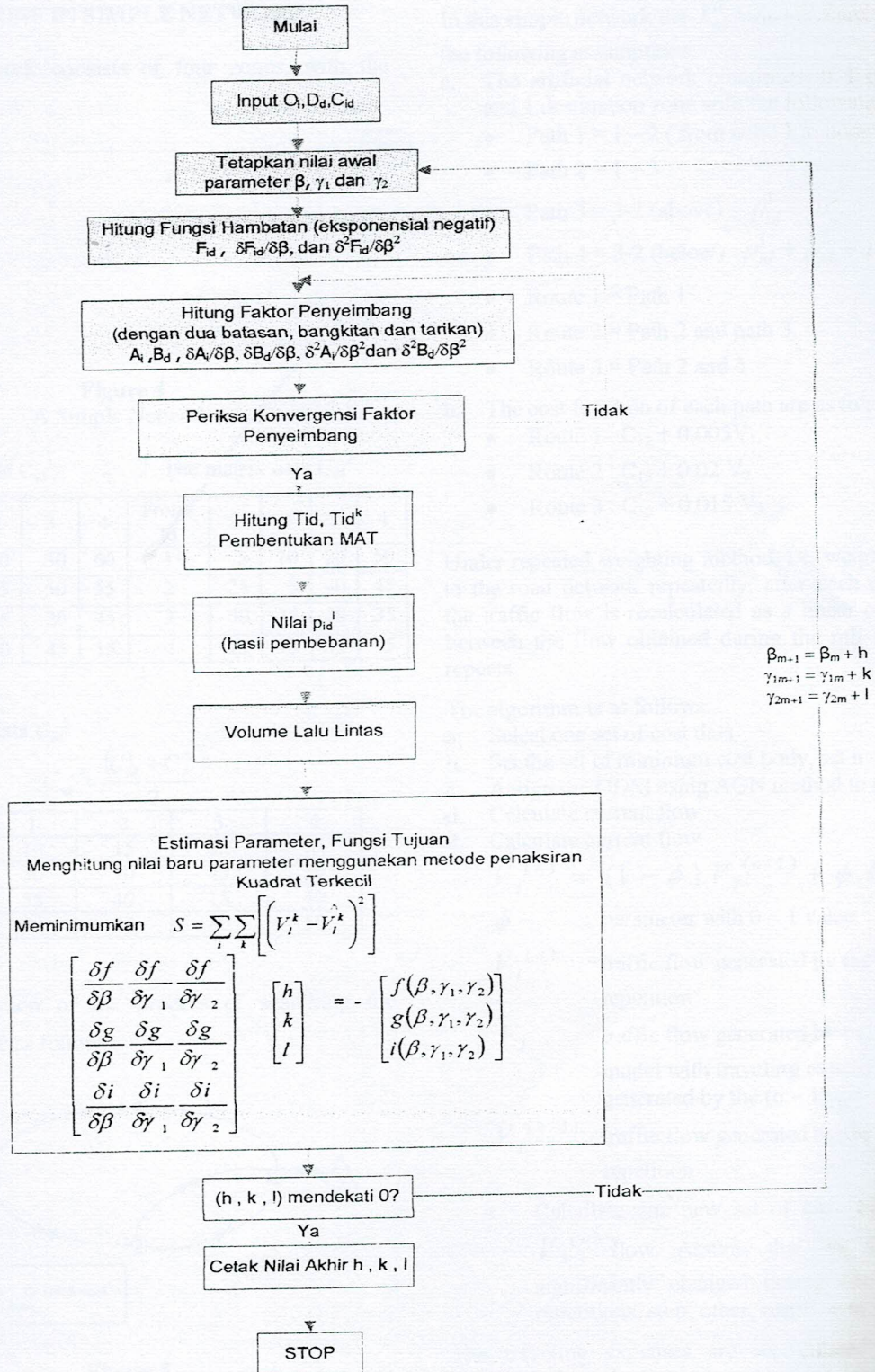
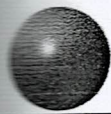
$$V_l^k = \sum_i \sum_d (O_i \cdot A_i \cdot B_d \cdot D_d \cdot f(C_{id})) \frac{\exp(-\gamma_k C_{id}^k)}{\sum_m \exp(-\gamma_m C_{id}^m)} p_{id}^{lk} \dots\dots (12)$$

Equation (12) is an equation system with L simultaneous equations having unknown  $\beta$  and  $\gamma_m$  parameters. To estimate the values of these parameters, certain estimation method is needed.

### VI. THE METHOD OF NEWTON-RAPHSON CALIBRATION AND GAUSS JOURDAN ELIMINATION METHOD

Systematically, the calibration process is illustrated in **Figure 3**.

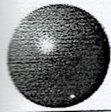




**Figure 3**

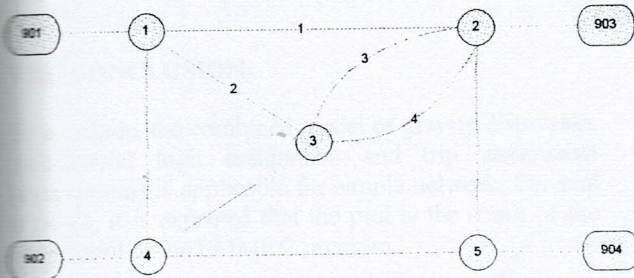
Flow Chart of calibration process for three unknown parameters





**VII. TRIAL RUNS IN SIMPLE NETWORK**

A simple network consists of four zones, with the following data:



**Figure 4**  
 A Simple Network

1. The matrix data  $C_{id}^1$       2. The matrix data  $C_{id}^2$

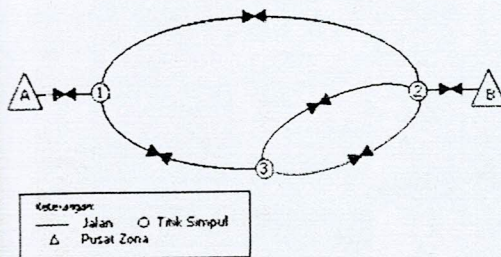
| From/<br>to | 1  | 2  | 3  | 4  | From/<br>to | 1  | 2  | 3  | 4  |
|-------------|----|----|----|----|-------------|----|----|----|----|
| 1           | 15 | 20 | 30 | 60 | 1           | 5  | 10 | 20 | 50 |
| 2           | 35 | 15 | 50 | 55 | 2           | 25 | 5  | 40 | 45 |
| 3           | 60 | 45 | 20 | 45 | 3           | 50 | 35 | 10 | 35 |
| 4           | 35 | 50 | 45 | 15 | 4           | 25 | 40 | 35 | 5  |

3. The matrix data  $C_{id}^3$

$$C_{id} = \frac{(C_{id}^1 + C_{id}^2)}{2}$$

| From/to | 1  | 2  | 3  | 4  |
|---------|----|----|----|----|
| 1       | 10 | 15 | 25 | 55 |
| 2       | 30 | 10 | 45 | 50 |
| 3       | 55 | 40 | 15 | 40 |
| 4       | 30 | 45 | 40 | 10 |

The simplification of the process of searching the values  $P_{id}^1$  are as follows:



**Figure 5**

The simplification of Network to obtain  $P_{id}^1$  values

In this simple network the  $P_{id}^1$  value is searched under the following assumptions:

- a. The artificial network comprises of 1 origin zone and 1 destination zone with the following paths:
  - Path 1 = 1 - 2 ( from node 1 to node 2)
  - Path 2 = 1 - 3
  - Path 3 = 3-2 (above)  $P_{id}^1 + P_{id}^2 = 1$
  - Path 4 = 3-2 (below)  $P_{id}^1 + P_{id}^3 + P_{id}^4 = 1$
  - Route 1 = Path 1
  - Route 2 = Path 2 and path 3
  - Route 3 = Path 2 and 4
- b. The cost function of each path are as follows:
  - Route 1 :  $C_{12} + 0,005V_1$
  - Route 2 :  $C_{12} + 0,02 V_2$
  - Route 3 :  $C_{12} + 0,015 V_3$

Under repeated weighting method, i.e. weighting ODM to the road network repeatedly, after each assignment, the traffic flow is recalculated as a linear combination between the flow obtained during the nth and (n-1)th repeats.

The algorithm is as follows:

- a. Select one set of cost data
- b. Set the set of minimum cost body, set  $n = n + 1$
- c. Assign the ODM using AON method to generate  $F_1$
- d. Calculate current flow
- d. Calculate current flow

$$V_i^{(n)} = (1 - \phi) \cdot V_i^{(n-1)} + \phi \cdot F_i$$

$\phi$  : parameter with 0 - 1 value

$V_i^{(n)}$  : traffic flow generated by the n<sup>th</sup> repetition

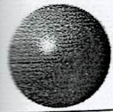
$F_1$  : traffic flow generated by the AON model with traveling expenses generated by the (n - 1) repetition.

$V_i^{(n-1)}$  : traffic flow generated by the (n - 1)<sup>th</sup> repetition

- e. Calculate one new set of cost, based on the  $V_i^{(n)}$  flow. Assume that this flow is not significantly changed during two sequential repetitions, stop; other, continue to stage (2)

The traveling expenses are recalculated after each combination of  $V_i^{(n)}$  flow has been assigned. This





process should be performed repeatedly until the convergence limit has been achieved.

The  $\Phi$  value = the inverse of the repetition numbers =  $1/n$

**VIII. CONCLUSION**

To conclude, the combined model of gravity dispersion, multinomial logit assignment and trip assignment (equilibrium) is applicable for simple network. For real network, it is expected that the pidl is the result of the assignment of the EMME/2 program.

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**ATTACHEMENT**

```

C:\WINDOWS\system32\cmd.exe
Masukkan Jumlah Zona: 4
Masukkan Nilai Beta: 0.06

Matrik Cidm <Moda 1>
10.00000 20.00000 10.00000 60.00000
15.00000 15.00000 50.00000 15.00000
30.00000 45.00000 20.00000 45.00000
35.00000 50.00000 45.00000 15.00000

Matrik Cidm <Moda 2>
5.00000 10.00000 20.00000 50.00000
25.00000 5.00000 40.00000 45.00000
10.00000 15.00000 10.00000 15.00000
20.00000 40.00000 15.00000 5.00000

Matrik Cid <Moda 1>
10.00000 15.00000 25.00000 55.00000
30.00000 10.00000 45.00000 50.00000
55.00000 40.00000 15.00000 40.00000
30.00000 45.00000 40.00000 10.00000

Matrik ITA
0.54881 0.40657 0.22313 0.03688
0.16530 0.54881 0.06721 0.01979
0.03688 0.09072 0.40657 0.09072
0.16530 0.06721 0.09072 0.54881

Nilai Bd Konvergen:
0.00000 0.00000 0.00000 0.00000

Nilai Pd Konvergen:
0.70326 0.89939 4.31778 0.99305

Nilai Tid:
1582.07244 2286.90519 956.74505 164.27535
350.68190 2271.83648 214.28873 163.19285
324.83659 1559.97920 5387.74096 1234.44324
1242.40714 1382.22812 1487.22422 8770.08816

Nilai Tidk <Tid Moda k>
550.59738 810.35079 342.56097 58.20993
124.26192 805.01093 75.93187 57.82636
115.10380 552.41445 1906.98597 437.41718
617.41168 489.80154 509.27134 3467.15532

Nilai do Tid:
4.48812 0.09054 1.57025 2.02817
4.95897 5.48812 3.02425 2.48735
2.02857 -3.62872 -0.09854 -3.62872
4.95897 -3.02425 -3.62872 -5.48812

Nilai do Tricj:
67.45600 81.22722 138.47493 402.70233
Nilai do Bicj:
-0.01448 -0.01426 -0.01200 -0.01023
Nilai do Tidci:
12547.39506 10156.56272 -14193.84694 -8509.11184
    
```





```
C:\WINDOWS\system32\cmd.exe
Nilai do_Tid(i)(j):
12547.39606 10156.56272 -14194.84694 -8509.11184
-4483.37376 19822.82320 -7585.57306 -7753.87638
-2657.21105 -11009.99172 47431.39722 -28764.19445
-406.81126 -18969.39425 -25650.97727 45027.18277

Nilai do_Tidk(i)(j):
826.55833 -1633.16701 -7241.61711 -3390.98623
-2390.96014 1826.48855 -3178.15890 -3120.89672
-3456.45938 -7468.01986 4494.44131 -13016.62251
-4130.50042 -9884.11974 -12377.40454 -4493.83976

Nilai do do_Ai(j):
16439.94037 111369.08216 156664.49943 134830.16451

Nilai do do_Bd(j):
2.13278 2.12942 1.95125 1.80759

Nilai do do_Tid(i)(j):
30882603.64529 44038921.80513 20159211.54760 4029198.06394
42488225.12239 274554564.93093 26449256.91319 20503484.42459
36974122.44935 176317099.62314 616345906.86580 143439447.14966
227389594.79386 180347316.91534 189715139.39188 1188005571.26704

Nilai do do_Tidk(i)(j):
19295670.98289 21356322.04487 -18374987.68699 43736867.57695
16708120.43361 118015096.38168 19251525.24220 24589105.20556
92338255.71888 87267413.81963 336982925.17437 408739728.99439
83566759.64277 127039234.82612 118669445.50786 527716165.01848

Arus akhir pada rute 1: 502.41749
Arus akhir pada rute 2: 129.65613
Arus akhir pada rute 3: 162.07016

Nilai pid tiap l:
0.63265 0.36735 0.16327 0.20408

Nilai f: -3137486.01322
Nilai f: -41030965620.56481

Nilai h: 0.00000
Nilai Beta baru adalah (Beta*(1+h)): 0.06000
Press any key to continue
```